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8. Olympic Saddledome, Calgary (Alberta)

Owner: Calgary Olympic Coliseum Society

Project Manager: Bill Pratt / Al Olson

Graham McCourt Architects, Architect:

Calgary

Civil and Structural

Jan Bobrowski and Partners Ltd., Engineer:

Calgary

Construction

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Manager: Cana Construction Co. Ltd. Precast Concrete: Genstar Structures Limited

Construction

Time: Autumn 1981 to Autumn 1983

October 1983 Service Date:

'Olympic Saddledome' provides a distinctive identity for Calgary. Meticulously planned and beautifully executed, it is a superb combination of form and function. Known as the Saddledome because of its double curvature roofline, the 20'000 seat stadium has been built as the flagship of the facilities for the 1988 Winter Olympic Games as well as the future home of the Calgary Flames of the National Hockey League.

The concept of the Saddledome was evolved from a sphere intersected by a hyperbolic paraboloid surface that forms the roof and by a flat plane that forms the base. The curve of the intersection between the sphere and the hyperbolic paraboloid defines the ring beam for the roof. The total volume of the Saddledome, by adopting this form, is indeed about 50% smaller than the average American coliseum yet it will accommodate, in comfort, almost 3'000 more spectators and make significant contribution to energy conservation. The shape of the hyperbolic paraboloid roof correlates closely with the sightlines.

The development of a concept from the spherical shape enables the rational use of prefabrication, particularly precast concrete. The external columns of the Saddledome, following paths of identical meridians, have all been cast in a single form. Internally, wide use was made of standard double tee floor elements and standard bleachers involving more than 1000 repetitions.

The extensive use of semi-lightweight and extra lightweight concrete enabled the lifting of large elements by cranes of limited capacity, as well as practical resolution of problems arising from uniquely large and rapid temperature oscillations characteristic to Calgary's cli-

The roof structure consists of two sets of cables forming a rigid network of $6 \text{ m} \times 6 \text{ m}$ grid on top of which 391 precast concrete roof panels of extra lightweight concrete are placed. The sagging cables are forced to remain parallel to each other and dip about 14 m in the centre. The hogging cables are laid on top of the sagging cables and are permitted to assume their free geodesic shape, ensuring perfect meshing of the 6 m square precast panels. Stressing is carried out in two stages. After the first stage stressing, precast panels are placed over the cables. After the second stage, the narrow strip between the precast panels is filled with cast-in-place concrete that turns the roof into a solid thin shell. No falsework is required for any operation.

To contend with deflections caused by wind and snow loads as well as extreme temperature fluctuations, the roof is designed to float on top of circular external columns by means of multidirectional bearings which transfer only vertical loads. The only place where the roof is fixed is at the two low points where rigid A-frames in pairs are used to resist the lateral loads from the roof. The ring beam at high points can move horizontally up to ±150 mm. Four bearings at each high side are also designed for uplift force.

Monitoring of deflections of the roof structure carried out during each stage of construction indicated an extremely close correlation between the actual field measurements and the theoretical analyses.

(S.S. Bhatia)



Fig. 5 Calgary Olympic Saddledome



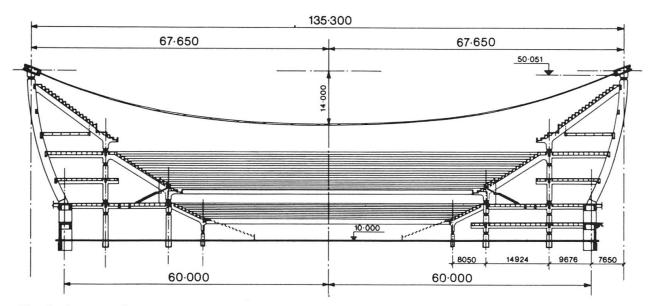


Fig. 1 Cross section

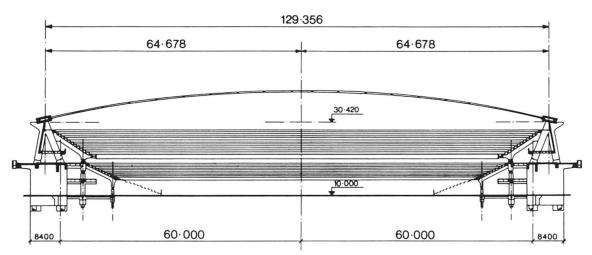


Fig. 2 Cross section

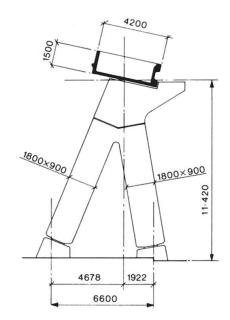


Fig. 3 Typical "A" frame elevation

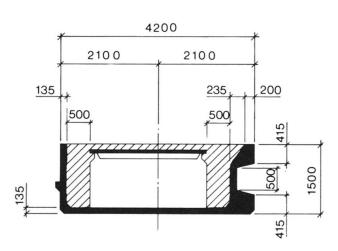


Fig. 4 Typical cros section thru ring beam